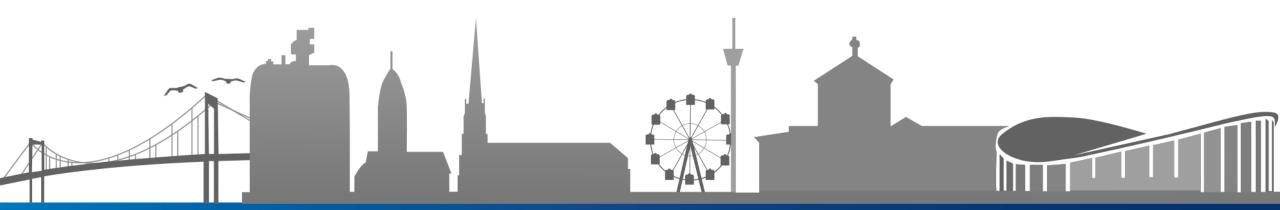


SwedCOLD Workshop 3: Best Practices for Tailings Dam Breach Analysis



Disclaimer



- This presentation is intended to be shared with the participants of the Workshop 3: "Best Practices for Tailings Dam Breach Analysis" held on June 14th 2023 at ICOLD's annual meeting in Gothenburg.
- We kindly ask to not share this document with third parties.
- Some graphical content originally used to illustrate case studies have been removed to avoid misuse of information.

Welcome!



Agenda

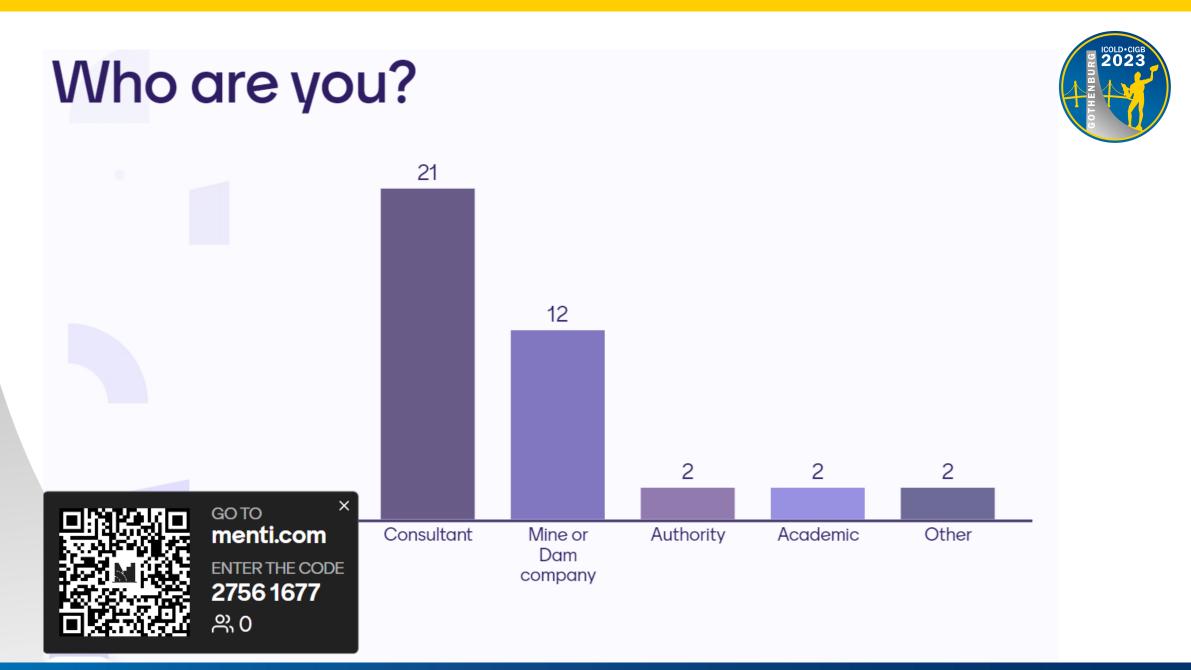


- Introduction, presentation of GruvRIDAS and guidance document no 3 ("TV3") on Tailings Dam Breach Analysis and consequence classification
- Workshop on Tailings Dam Breach Analysis:
 - Part 1 Case study of different tailings dam breach cases (1A-1B-2A-2B)
 - Part 2 Group discussions on chosen topics
- Round-up and conclusion
 - Summary of group discussions
 - Conclusion

Who are you?



- Who are you?
- Join at menti.com use code 2756 1677





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- M.Sc. Mechanical engineering 1999-2004 (Lund, Sweden)
- Team manager Dam safety at LKaB
- Tailings Governance
- 10 years of tailings management at LKAB
- 6 years as consultants in both hydro power sector and mining sector
- Chair of the Swedish dam safety group within (Svemin) the mining sector for 5 years



Hans Häggström Boliden hans.haggstrom@boliden.com



- M.Sc. Civil Engineering 2005-2010 (Luleå, Sweden)
- Geotechnical engineer
- Dam safety and tailings management
- Dam safety controller / RIDAS-ansvarig
- Dam safety review, geotechnical investigation and stability calculations (2011-2017, Ramböll)



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- M.Sc. Hydraulic and Geotechnical engineering 2002-2006 (Grenoble, France)
- Specialist Hydraulic engineering
- Numerical modelling
- Dam & Tailings Dam Breach Assessments
- Dam safety
- Hydrological design



Anna Risberg Sweco <u>anna.risberg@sweco.se</u>



- M.Sc. Aquatic and Environmental engineering 2000-2006 (Uppsala, Sweden)
- Team manager Hydraulics and dams
- Dam safety, hydraulics and consequence classification
- Dam safety supervision at County Administrative Board 2019-2022



Mohammad (Mamun) Al-Mamun Tetra Tech

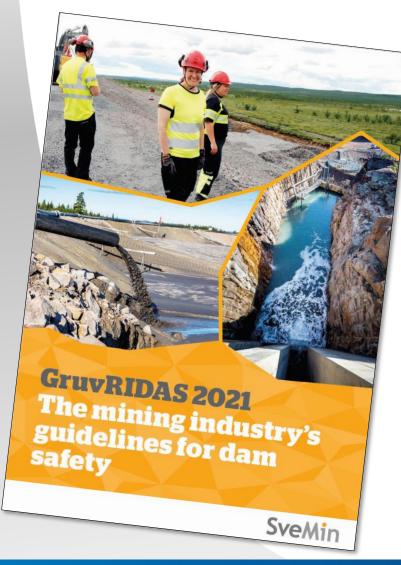
Mamun.almamun@tetratech.com



- M. Sc. Eng. Geotechnical Engineering, 2001-2003 (Univeristy of Calgary, Canada)
- Principal Consultant Dam Practise
- Design, construction, operation, and rehabilitation of dams (hydro and tailings)
- Mining waste and tailings management; mine closure and reclamation, tailings dam breach analysis (TDBA)
- Dam safety management and risk assessment
- Co-chair, Tailings Dam Breach Working Group, Canadian Dam Association (CDA.ca)
- Member, Technical Committee on Climate Change Adaptation of Dams, Canadian Standard Association (<u>CSAgroup.org</u>)

1 Introduction

National guidelines for dam safety



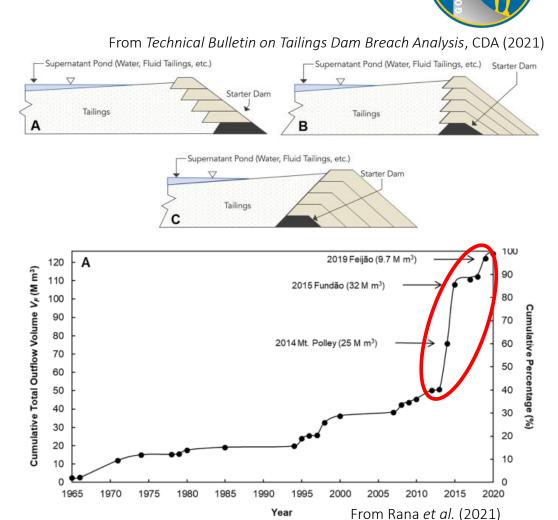
- 2021 third edition of GruvRIDAS.
- Ambition to guide Swedish mining company to compliance according to Swedish legislation.
- 1 main document and approximately 15 guidance document.
- Guidance document for "Dam breach assessment and dam safety classification" will be launched in 2023.
- Ambition to translate all documents to English.





Evolution of methods involved in TDBAs: overview

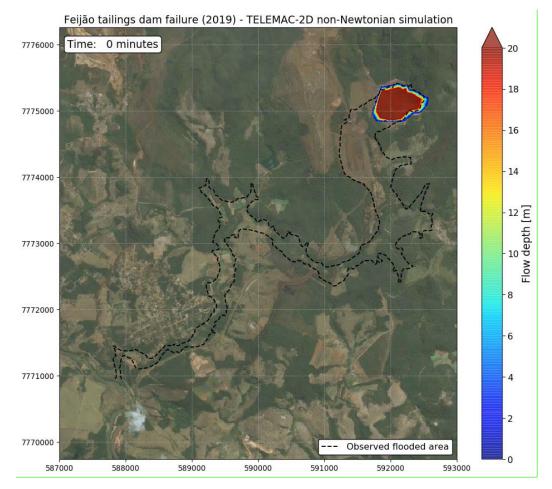
- Historically: mainly based on classic dam breach methodology for water retention dams
- TSFs are more complex:
 - Different media stored, complex geotechnical conditions
 - Variety of specific dam designs
 - Often, varying operating levels, dam raise operations
 - Tailings dam failures ~1.2% of total number of facilities (Azam & Li, 2010). ~ 100x than for water dams
- Lessons learned from failures:
 - Large variety of failure modes, overtopping / seepage not dominant as for water dams
 - Tailings liquefaction can dramatically influence the outflow volume and runout





Evolution of methods involved in TDBAs: overview

- Recent & ongoing developments:
 - Better understanding of tailings geotechnical properties to inform dam breach assumptions
 - New and dedicated dam breach modelling methods + probabilistic analysis of dam breach parameters
 - Improved runout modelling (1D -> 2D -> 3D/CFD), including non-Newtonian rheological models, geo-mechanical models
 - Widespread use of sensitivity analyses to estimate and communicate about uncertainties
 - Consequences estimation and mitigation (emergency response plans, Loss-of-life and evacuation modelling...)





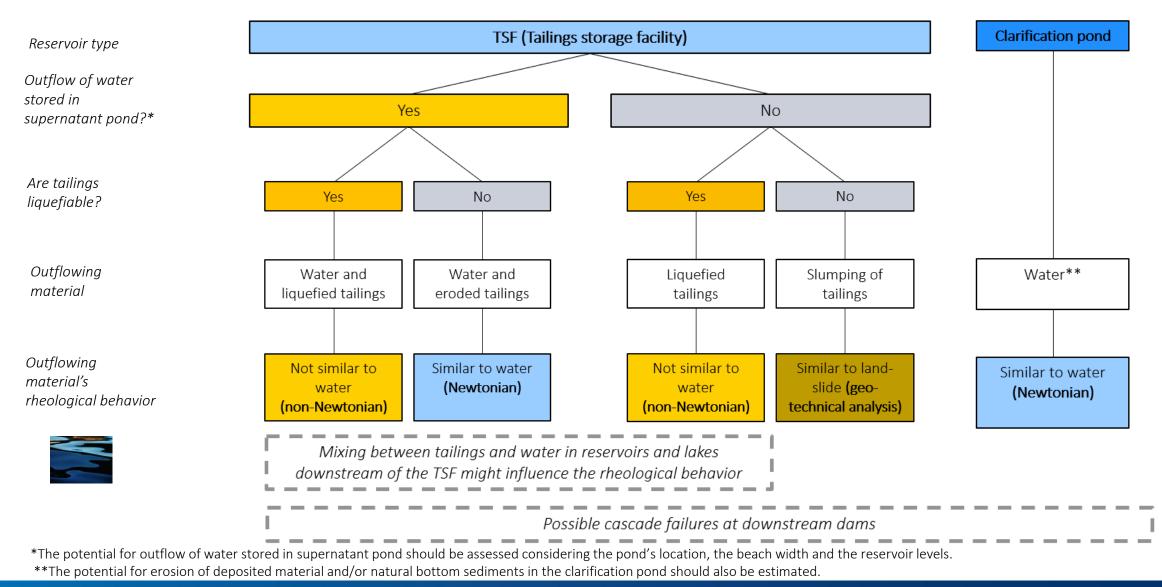
GruvRIDAS TV3 - The new guidelines for Tailings dam breach analysis and classification

1	1 INTRODUCTION			
2 ASSESSMENT OF CONSEQUENCES				
2.1	Introduction			
2.2	Prerequisites to analyze and describe the Tailings Storage Facility (TSF)			
2.3	Dam breach assumptions and scenarios			
2.4	Runout analysis			
2.5	Documentation specific to the TDBA			
2.6	Identification of objects within inundation area			
2.7	Assessment of consequences			
3 CLASSIFICATION				
3.1	Dam safety classifcation according to Swedish Environmental Code			
3.2	Decision on risk facility according to regulation on extractive waste			
3.3	Classification for specific contingency requirements			
3.4	Consequence classification according to Global industry standard on tailings management (GISTM)			

Assessment of severity of consequences for design flood determination

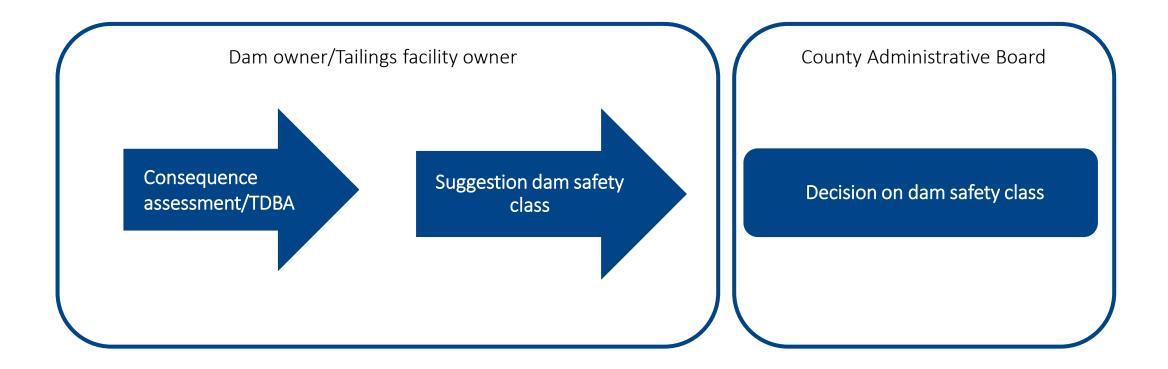


GruvRIDAS TV3 - The new guidelines for Tailings dam breach analysis and classification



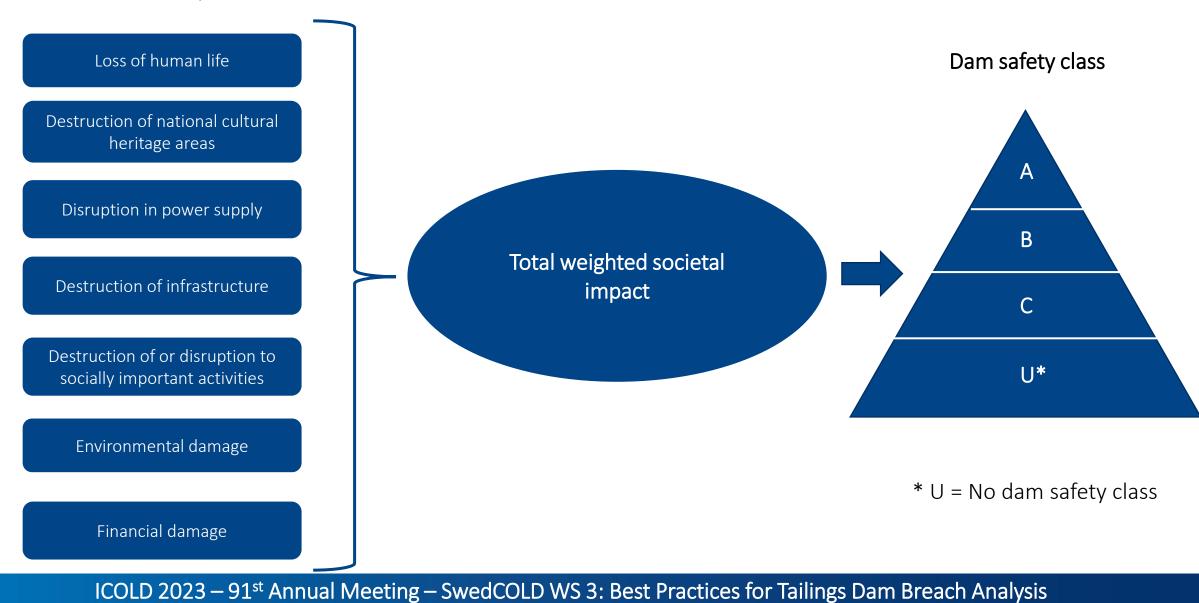
Dam safety classification in Sweden - process

• According to Swedish Environmental Code, 11 chapt. 24-26§§



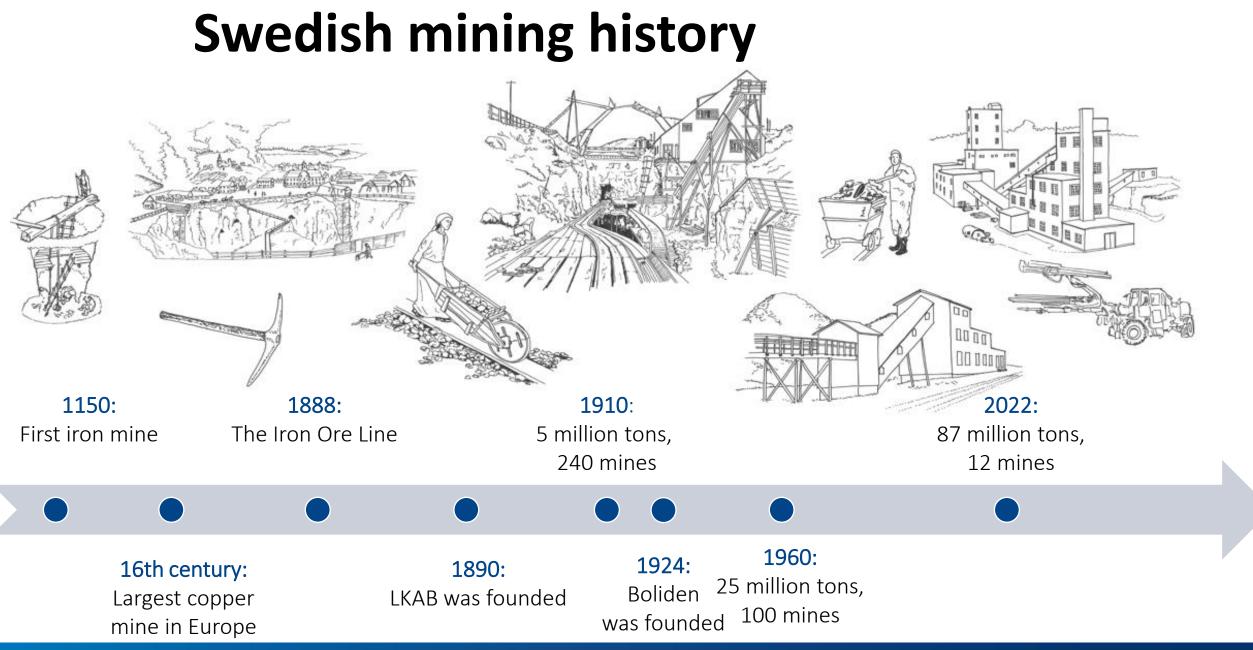
Dam safety classification in Sweden

Incremental consequences



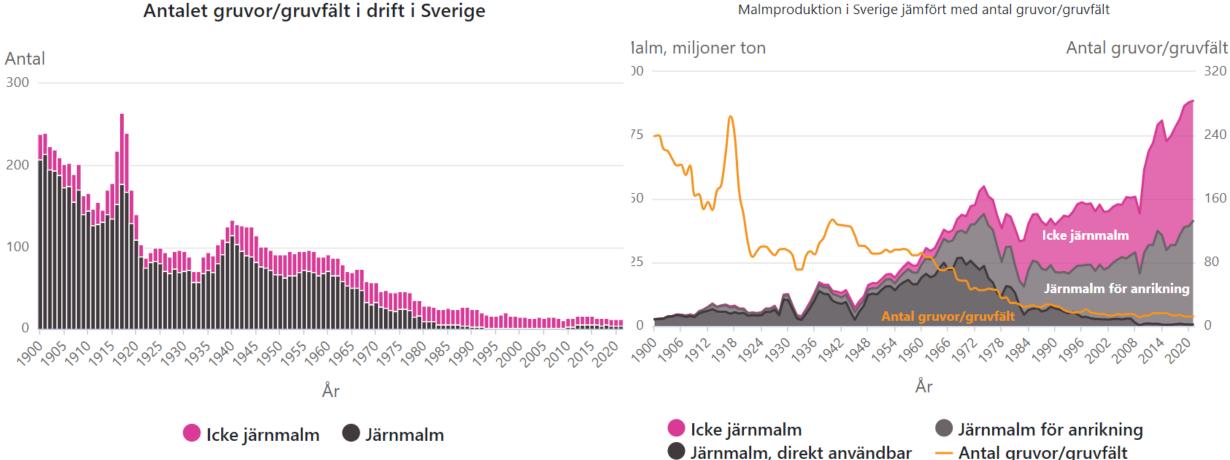
Dam safety classification in Sweden

Dam safety class	Severness/impact to society	Potential causes
Α	Very serious	 A national crisis affecting large numbers of people and large parts of civil society and threatening fundamental values and functions. Non-negligible risk of loss of human life.
В	Serious	 Major regional and local consequences or disruption. Non-negligible risk of loss of human life.
C	Moderate	 Significant local consequences and disruption, injuries on local infrastructure, damage to property or environmental damage, or temporary disruptions. Negligible risk of loss of human life.
U (No dam safety class)	Small/Low	 No significant consequences or disruptions. Minor and local damage that can be restored without excessive costs.



Swedish mining statistics

Malmproduktion





Development of Management Practice in Sweden

1960-1970



Peak of 2000 hydropower Several tailings Shift to upstream dams construction 1995 2022 9 TF in operation. 11 Tailings facilities in Shift to construction operation. with low tailings single/end pipe discharge dependency. point from natural ground



TDBA: Current State of Practice and Global Perspective

Current State of Practise



Pre-2010s'

- Not often done
- When done, tailings is considered as water
- No concerted effort to progress TDBA

2015-2020

- Growing awareness and interests
- Workshops, technical papers, forum style discussions
- Early initiatives to develop guidelines by CDA

2020- Present

- English version issued Sep. 2021 by CDA
- Numerous research program by universities
- Efforts by other groups to develop process, guidelines, etc.



- Limitations of water-based models are generally accepted by the industry
- More awareness to accurately analyze downstream impacts due to tailings dam breach

GISTM, August, 2020



- Requirements 2.3: Develop and document a breach analysis for the tailings facility using a methodology that considers credible failure modes, site conditions, and the properties of the slurry. The results of the analysis shall estimate the physical area impacted by a potential failure. When flowable materials (water and liquefiable solids) are present at tailings facilities with Consequence Classification of 'High', 'Very High' or 'Extreme', the results should include estimates of the physical area impacted by a potential failure, flow arrival times, depth and velocities, and depth of material deposition.
- Requirements 2.4: In order to identify the groups most at risk, refer to the updated *tailings facility breach analysis* to assess and document potential human exposure and vulnerability to *tailings facility credible failure scenarios*.



Credible Failure Mode



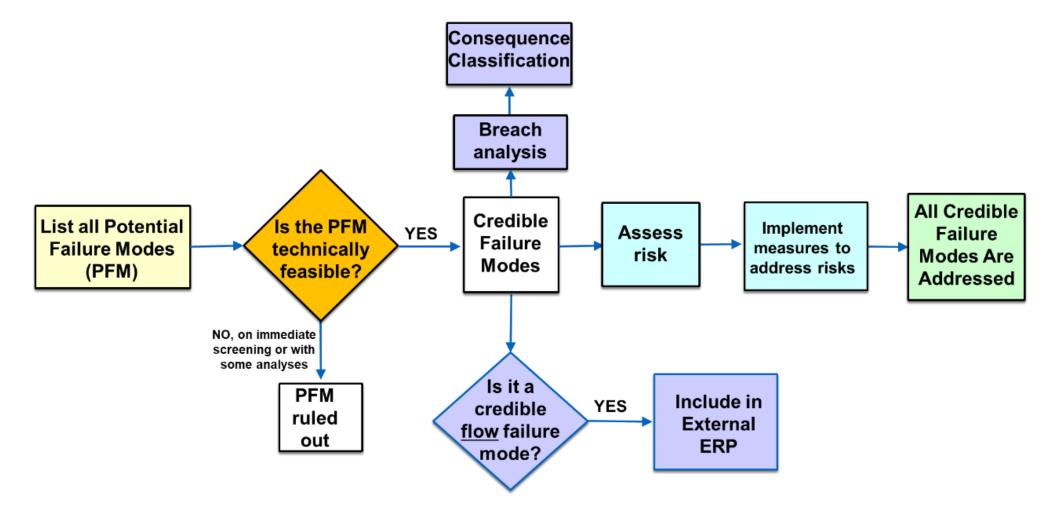
 Global Industry Standard on Tailings Management (GISTM, 2020) defines credible failure modes as follows:

<u>**Credible</u>**: Refers to **technically feasible failure mechanisms** given the materials present in the structure and its foundation, the properties of these materials, the configuration of the structure, drainage conditions and surface water control at the facility, throughout its lifecycle.</u>

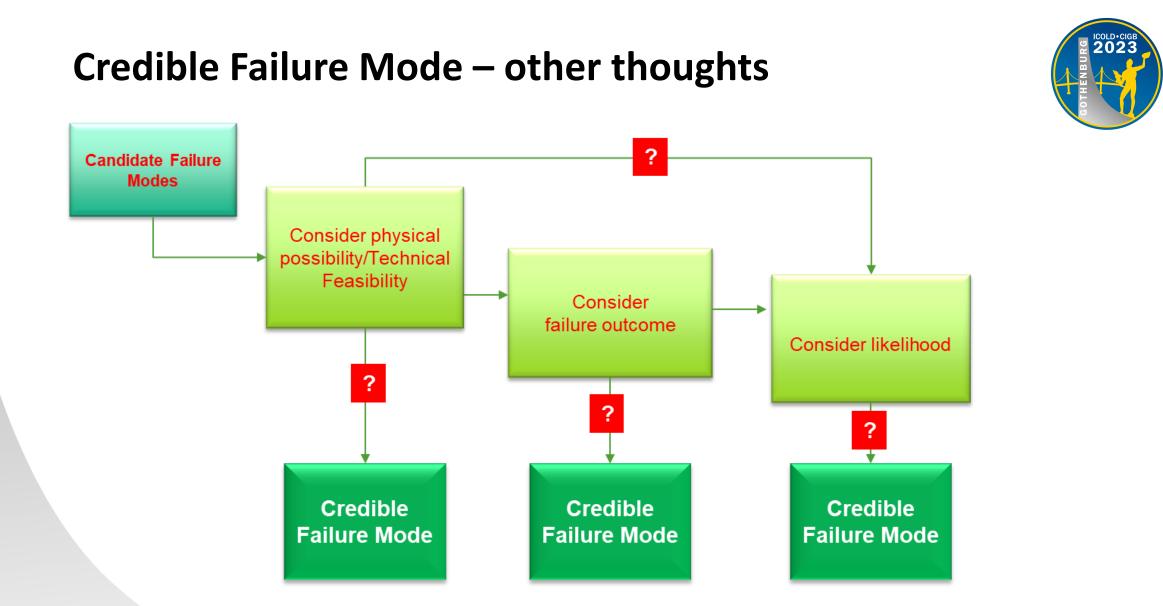
GISTM also noted:

- Credible failure modes can and do typically vary during the lifecycle of the facility as the conditions vary.
- Credible catastrophic failure modes do not exist for all tailings facilities.
- The term 'credible failure mode' is **NOT** associated with a **probability of this event** occurring and having credible failure modes is not a reflection of facility safety.

Credible Failure Mode – GISTM Flowchart



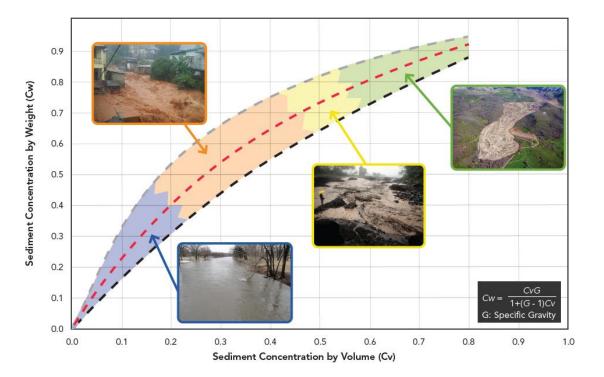
Prepared by: Angela Kupper for CFM workshop, Tailings and Mine Waste, 2021, Banff, Canada.



Credible Failure Modes Workshop, 2021 Tailings and Mine Waste Conference, Vancouver, Canada

Properties of Tailings

- Vast difference between different mineral tailings.
- In-situ rheology of tailings is relevant for tailings mobilization process.
- Rheology parameters tested for pipeline design or design of thickeners may not be suitable for TDBA.
- Information on tailings composition and characteristics are sourced from literature review due to the limitations testing data.



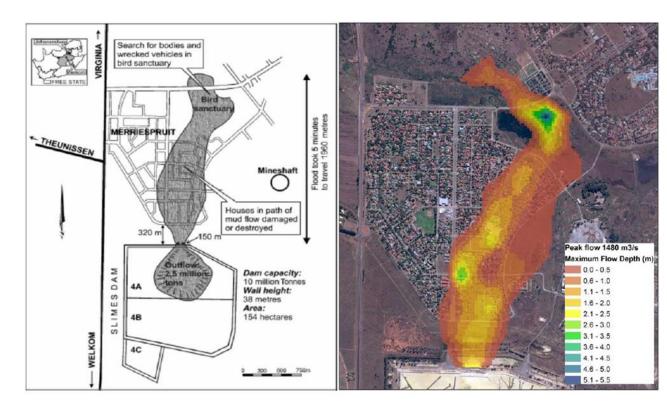
Technical Bulletin: Tailings Dam Breach Analysis, Canadian Dam Association, 2021.





Application of TDBA – Consequence and emergency planning

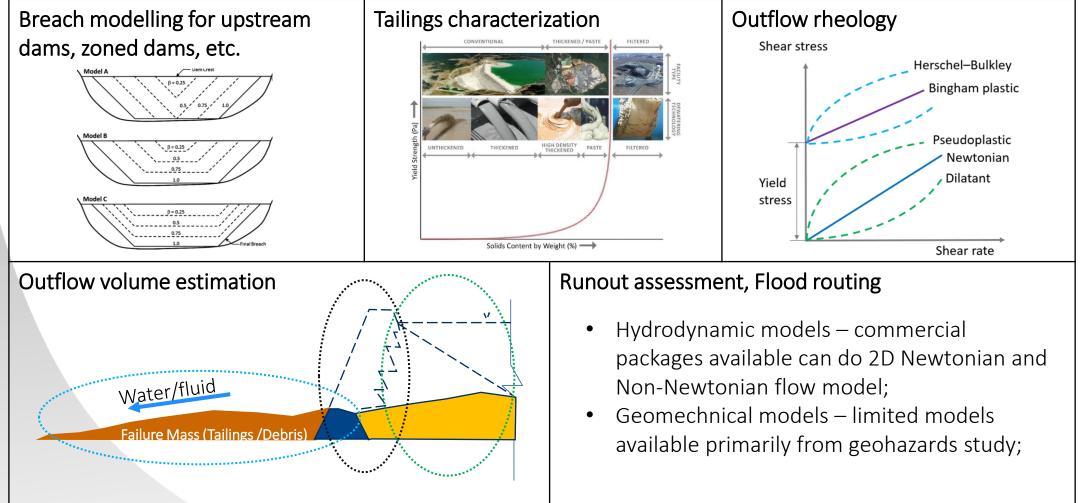
- For 'extreme' or 'very high' consequence TFS TDBA may not be required to establish design criteria
- But, for all TSF, TDBA is required to delineate inundation zones for emergency planning
- TDBA and inundations maps are useful tool for stakeholder engagement and risk communication



Tailings Dam Breach Workshop 2015, Canadian Dam Association

Challenges remain!





2 Workshop Part 1



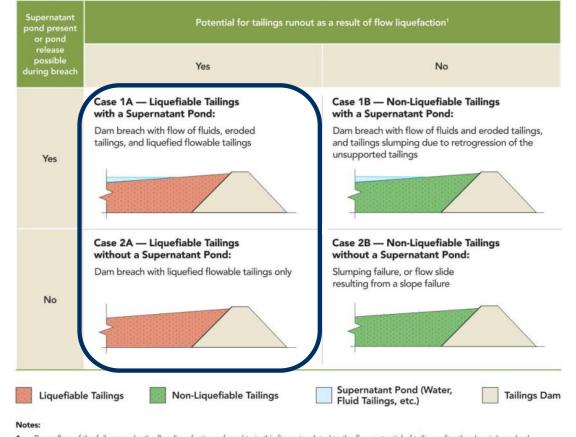
Part 1: Case study of different tailings dam breach cases

- Examples of cases involving each of the 4 tailings dam breach cases
 - 1A-2A: Example from Swedish mine (Garpenberg TSF, Boliden)
 - 1B-2B: Examples from historical & hypothetical cases
- Runout analysis:
 - Basic requirements
 - Challenges and limitations
- Consequences Example estimating potential loss of life
- Questions



Part 1: Examples of TDBA for Tailings dam breach cases 1A & 2A

- Cases 1A & 2A: liquefiable tailings with/without a pond
- TDBA study case: Garpenberg TSF
 - Permit application
 - Consequence classification according to Swedish Environmental Law and GISTM
 - Disclaimer: no generalization from this specific example



1. Regardless of the failure mode, the flow liquefaction referred to in this figure is related to the flow potential of tailings after the dam is breached.

From Technical Bulletin on Tailings Dam Breach Analysis, CDA (2021)



- Garpenberg TSF (Boliden)
- First mining operations started in 1200-1300
- Mine/TSF recent history:
 - Acquired by Boliden in 1957
 - First, deposition in lake
 - Deposition above water (spigotting) since 2007-2008
 - Dam construction methods:
 - 1960-2008: centerline
 - 2008-2022: upstream raised
 - 2023-2025: centerline with d/s rockfill embankment

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- Geotechnical conditions:
 - Tailings characterization study (2021)
 - CPT, shear vanes, triaxial (critical state tests), in-situ void ratio
 - Risk for liquefaction can't be excluded (loose deposits of contractive and strain softening silt tailings at depth)
 - Geotechnical properties highly dependant on deposition methods/levels:

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- Hydrological conditions:
 - Design flood: "Klass I" (AEP < 1:10,000)
 - Supernatant pond:
 - 0,2 Mm³ for normal operation
 - 0,5 Mm³ for design flood (spillway's passive design allows to account for flood evacuation) – very small additional volume w.r.t. tailings outflow
 - Hydrographic network:
 - Lakes and rivers located d/s all the considered breach locations

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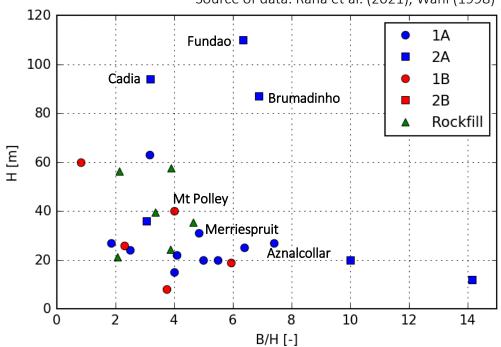


- Critical failure mode:
 - Foundation failure involving low-levels tailings
 - Static liquefaction: not credible (dam design can withstand it)
 - Internal erosion: can be considered as credible, but less critical than stability / foundation failure
 - Overtopping: not credible (freeboard + spillway's passive operating, risk for debris blockage minimal)

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- Breach dimensions:
 - Analysis of historical events and existing guidelines
 - Data from rockfill dam failures included
 - Considered breach ratio B/H: 3 to 5
 - Advanced numerical models can also potentially be used



Source of data: Rana et al. (2021), Wahl (1998)



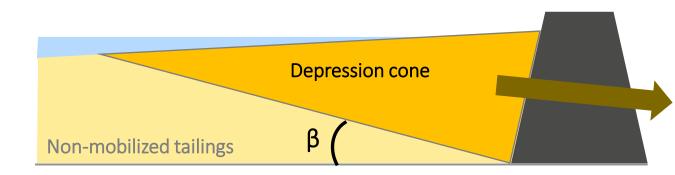
- Outflow volumes:
 - Water (supernatant pond, if relevant)
 - Liquefied tailings
- Case 1A or 2A depending on breach location w.r.t. supernatant pond
- Different outflow processes depending on breach location w.r.t. supernatant pond:
 - 1st Tailings 2nd Water
 - 1st Water 2nd Tailings

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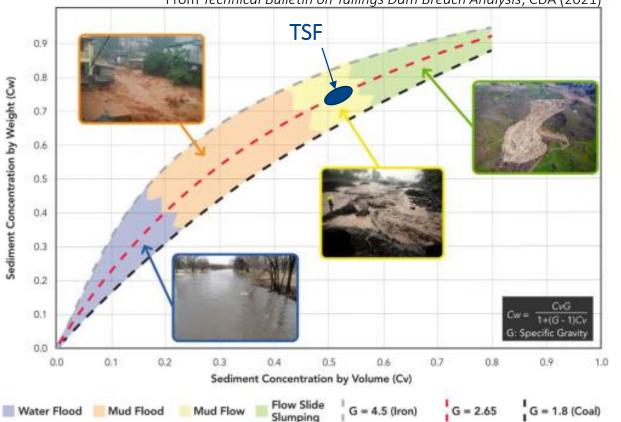
- Outflow volumes, tailings:
 - <u>Several methods investigated</u>
 - Determined by geometry of depression cone, most suitable (see previous slide)
 - Cone slope defined following hypothesis:
 - Tailings liquefy during outflow
 - Tailings can be considered saturated
 - Cone's slope can be estimated using Infinite slope theory from residual shear stresses (Seddon, 2007)
 - Slopes between 3 and 5 degrees were considered
 - Advanced numerical models can also potentially be used

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- Tailings rheology:
 - Outflow behaviour highly dependant ٠ on material concentration & degree of saturation
 - Tailings' volumetric concentration in • TSF estimated in range **Cv = 50-54%**
 - Rheological model used (runout):
 - Bingham ٠
 - Bulk density, viscosity, yield stress
 - Uncertaintes in rheological ٠ properties: two scenarios (span)
 - Mixing with d/s water bodies affects the rheological behaviour



From Technical Bulletin on Tailings Dam Breach Analysis, CDA (2021)



- Runout analysis:
 - 2D hydrodynamic models
 - Non-Newtonian (Bingham) & Newtonian rheology with mixing effects accounted for
 - Outflow processes:
 - 1st Tailings 2nd Water
 - 1st Water 2nd Tailings
 - Handling of both Non-Newtonian (Bingham) and Newtonian behaviour based on local Cv
 - Casacade failure in d/s dams or infrastructure

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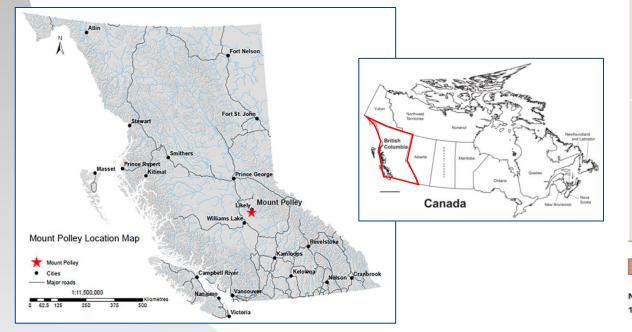


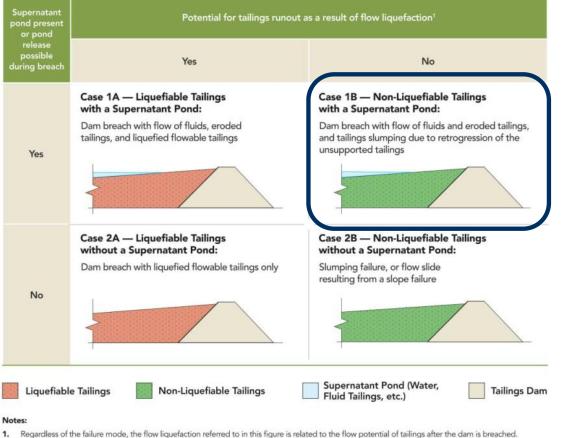
• Sensitivity analysis:

Scenario	Outflow volume (i.e. slope of erosion cone)		Rheological parameters		Breach development time	
	3 degrees	5 degrees	Low	High	Short	Long
1	0		0		0	
2	0			0		0
3		0	0		0	
4		0		0		0



- Mount Polley TSF (open pit copper and gold mine)
- Located British Columbia, Canada





From Technical Bulletin on Tailings Dam Breach Analysis, CDA (2021)

https://www.cbc.ca/news/canada/british-columbia/mount-polley-spill-blamed-on-design-of-embankment-1.2937387

- Operation since 1995
- Dam Height: ~40 m
- Surface Area : ~2.2 km²
- Storage:
 - Pond Water ~ 10.6 Mm^3
 - Copper, Gold, Silver Tailings ~63.3 Mm³
- Failed on August 4, 2014
 - Pond presence? Liquefiable tailings

Independent Expert Engineering Investigation and Review Panel. 2015. Report on Mount Polley Tailings Storage Facility Breach, January 30, 2015 (https://www.mountpolleyreviewpanel.ca/sites/default/files/report/AppendixC_SurfaceInv estigation.pdf)



https://en.wikipedia.org/wiki/Mount_Polley_mine#/media/File:Mount_Polley_Mine_site.jpg



- Failure Mode:
 - Climate condition: Fair Weather
 - Triggers of the failure:
 - Weak foundation layer
 - Overly steep dam slope
 - Excavation near the dam toe
 - Mismanagement (pond close to the crest, insufficient freeboard)
 - Failure sequence
 - Phase 1 Dam crest deformation
 - Phase 2 Overtopping failure



Geotechnical Investigation of the Mount Polley Dam - KCB (klohn.com)

Part 1: Examples of TDBA for Tailings dam breach cases 1B Breach Location Mount Polley LOCATION OF SECTION BY AMEC IN 2011 EMBANKM Dam Cross-Section at EMBANKME SOUTH **Breach Location** CORE (ZONE S) - EI. 969 m 1.016 ROCK (ZONE C) - El. 969 m 975 TAILINGS - EL 967.4 M 970 965 960 955 950 POST-BREACH GROUND 945 PROFILE (AUGUST 5, 2014) 940 WHALEBACK 935 930 925 920 915 Quesnel LOWER TILLS 910 Lake 905 900 895 890 885 Hazeltine Creek BEDROCK Source: https://earthobservatory.nasa.gov/images/84202/dam-breach-at-mount-polley-mine-in-british-880 875 -80 columbia 120 20 100

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Source: Report on Mount Polley Tailings Storage Facility Breach, 2015

- Release Volume estimate:
 - Pond water:
 - ~10.6 Mm³
 - Mobilized Tailings:
 - ~13.8 Mm³
 - No noticeable flow liquefaction

TABLE C4.5.2: BULK TAILINGS AND WATER VOLUMES

MATERIAL/PROPERTY	PRE-BREACH CONTENTS (million cubic meters)	CONTENTS RELEASED (million cubic meters)	
FREE WATER	10.6*	10.6	
BULK TAILINGS	63.3	13.8	
TOTAL	73.9	24.4	



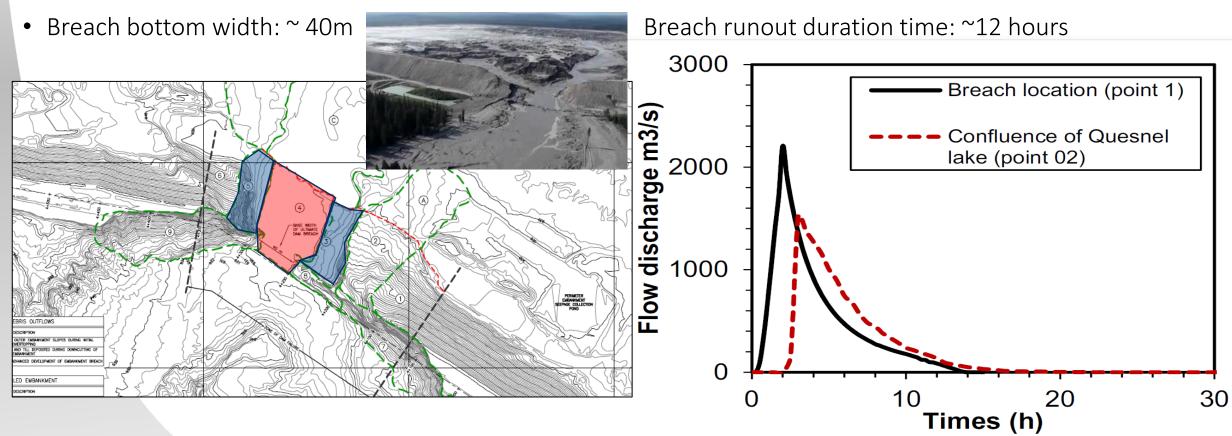
Klohn Crippen Berger. 2015. Mount Polley tailings dam failure – Assessment of failure mechanism. Report No. M09954A01.730. BC Ministry of Energy and Mines, May 2015.

https://www.mountpolleyreviewpanel.ca/sites/default/files/report/AppendixC_SurfaceInvestigation.pdf



• Breach height: ~40m

• Breach side slope (H:V): ~1:1



Klohn Crippen Berger. 2015. Mount Polley tailings dam failure – Assessment of failure mechanism. Report No. M09954A01.730. BC Ministry of Energy and Mines, May 2015.

Mahdi, A., Shakibaeinia, A., Dibike, Y.B., 2020. Numerical modelling of oil-sands tailings dam breach runout and overland flow. Sci. Total Environ. 703, 134568. https://doi.org/10.1016/J.SCITOTENV.2019.134568.



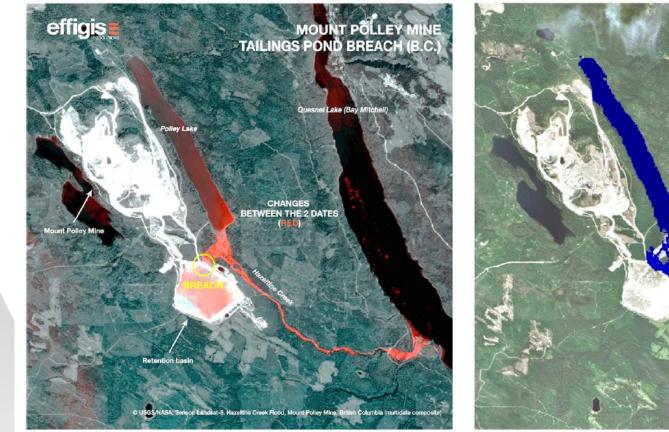
Runout Model:

- Total volume of outflow was used assuming non-Newtonian flow
- Measured water level data from the lake at the downstream was used to calibrate rheology parameters
- Commercially available software Flo-2D was used (Mahdi and Dibike, 2020^{*})

*Mahdi, A., Shakibaeinia, A., Dibike, Y.B., 2020. Numerical modelling of oil-sands tailings dam breach runout and overland flow. Sci. Total Environ. 703, 134568. https://doi.org/10.1016/J.SCITOTENV.2019.134568.



(a) Satellite Imagery



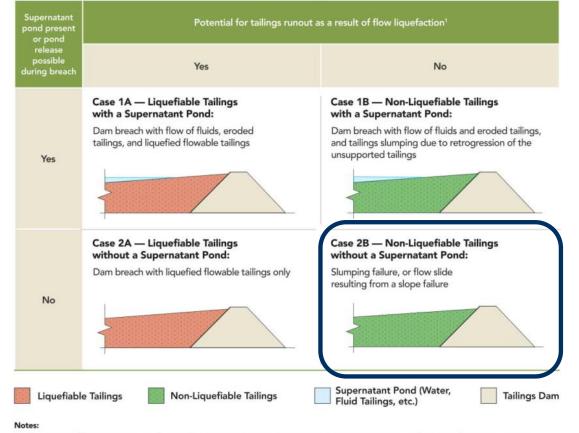
(b) Modelling result - inundation extent



Mahdi, A., Shakibaeinia, A., Dibike, Y.B., 2020. Numerical modelling of oil-sands tailings dam breach runout and overland flow. Sci. Total Environ. 703, 134568. https://doi.org/10.1016/J.SCITOTENV.2019.134568.



- Two hypothetical cases presented
- Both are closed sites
 - One site assumed postearthquake failure condtion
 - Other site assumed foundation shearing at normal conditon

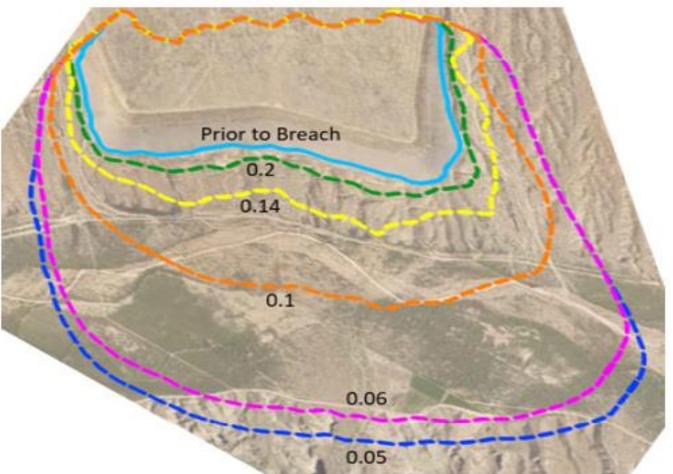


From Technical Bulletin on Tailings Dam Breach Analysis, CDA (2021)

1. Regardless of the failure mode, the flow liquefaction referred to in this figure is related to the flow potential of tailings after the dam is breached.



- Closed TSF with no liquefiable tailings stored
- It assumed entire dam can be breached
- Post-earthquake factor of safety was assumed less than 1.0 to initiate a hypothetical failure.
- Sensitivity analysis conducted on runout extent using a range of residual undrained shear strength ratios.



CDA 2021 Annual Conference (Chen et al. 2021)



- TSF with no liquefiable tailings stored
- Two breach locations assumed
- For normal condition there is no water on the TSF
- Can only fail due to foundation shearing or shearing through the dam

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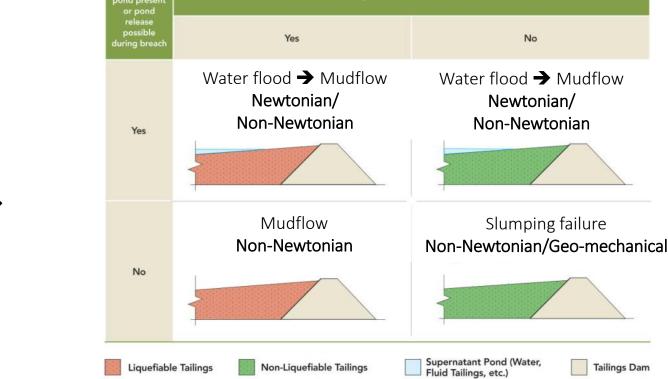


- Assumption for runout assessment
 - Rheology: frictional material
 - Erosion: no
 - Unit Weight: 20 kN/m3
 - Internal friction angle of tailings: 35 degree
 - Friction angle of the sliding surface: 5.7 degree (equivalent to su/sigv = 0.1)

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Part 1: Runout analysis, basic requirements

- Type of analysis based on expected rheological behaviour (liquefaction potential, value of Cv)
- Importance of outflow processes and their order (water pond, tailings)
- For outflow of types water flood → mudflow, 2D hydrodyamic models with Newtonian & non-Newtonian rheological models are adapted
- Use of geomechanical models required for slumping failures



1. Regardless of the failure mode, the flow liquefaction referred to in this figure is related to the flow potential of tailings after the dam is breached.



Potential for tailings runout as a result of flow liquefaction

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upernatan





Part 1: Runout analysis, challenges & limitations

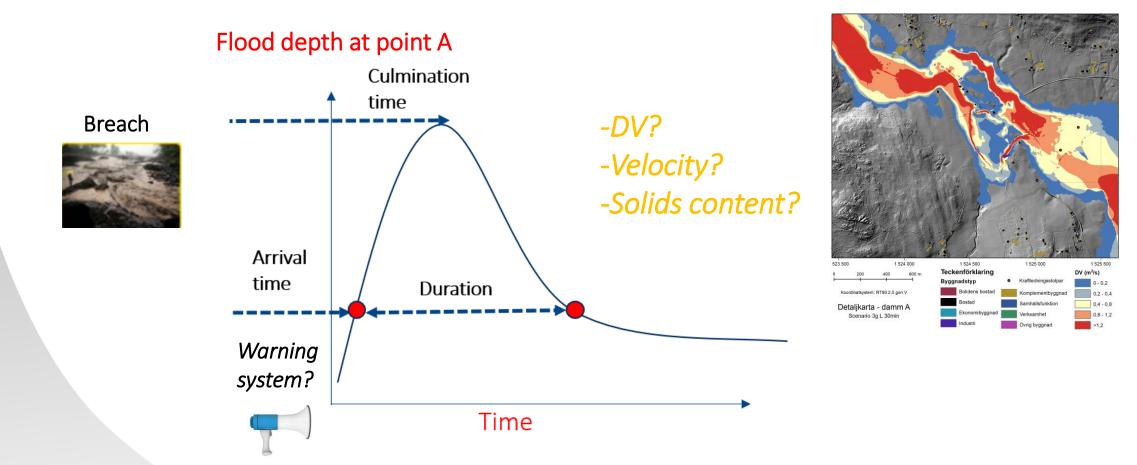
- Representative tailings rheology for dam breach runout is difficult to estimate, large uncertainties
- Actual flow characteristics & processes are very complex VS relatively simple rheological models used (e.g. Bingham)
- Mixing with d/s water bodies can dramatically influence rheology, runout distance & deposition (e.g. Fundão, 2015, > 600 km)
- Importance of appropriate model type and of sensitivity analysis to assess range of consequences







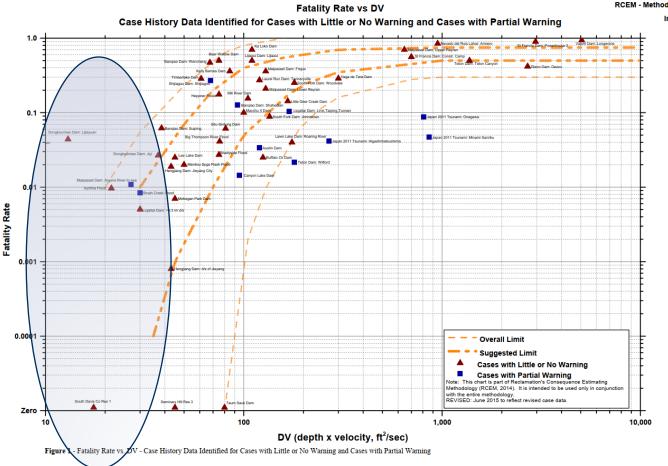






• RCEM Methodology (USBR, 2014)

- Database with world-wide case histories of fatalities caused by floods/breaches in water dams
- Two charts:
 - "Little or no warning"
 - Warning is considered to be "adequate".
- Observed fatality rates are specific to the warning scenario, time of day and year, etc of the incident
- Uncertainty range is increasing for the lowest flood severity values (DV values < approx. 5 m2/s)





- Flood hazard assessment method (ARR, 2010)
- Limiting values for unsafe conditions (ARR, 2019):
 - depth of 1,2 m (children and elderly) or 2 m (adults)
 - velocities of 2 m/s (adults, children and elderly)

Table 6. Interval of DV values and the corresponding hazard for different age groups (ARR, 2010). "H.M" corresponds to the product of Height (m) by Mass (kg).

DV (m ² s ⁻¹)	Infants, small children (H.M ≤ 25) and frail/older persons	Children (H.M = 25 to 50)	Adults (H.M > 50)
0	Safe	Safe	Safe
0-0.4		Low Hazard	
0.4 - 0.6	Extreme Hazard; Dangerous to all	Significant Hazard; Dangerous to most	Low Hazard ¹
0.6 - 0.8			Moderate Hazard; Dangerous to some
0.8 - 1.2		Extreme Hazard; Dangerous to all	Significant Hazard Dangerous to most
> 1.2			Extreme Hazard; Dangerous to all



• Ex debris flows and fluidized landslides, Pollock and Wartman, 2020

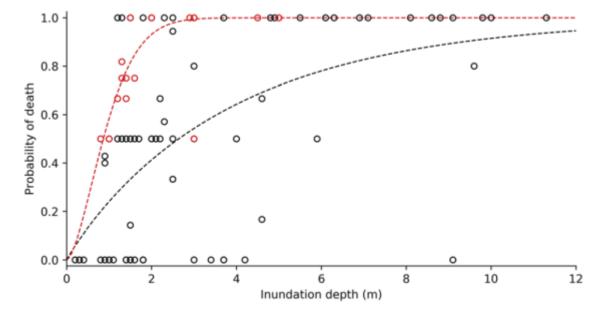


Figure 11. Fatality rate depending on flow depth for debris flows and fluidized landslides in economically developing (red curve) and developed (black curve) countries (Pollock and Wartman, 2020).

3 Workshop Part 2

Appointed group leaders
 Prepare:

- 1. Summary for big group
- 2. Summary of discussions at Microsoft Forms

SwedCOLD Workshop 3: Best Practices for Tailings Dam Breach Analysis







1. Dealing with uncertainties and simplifications

Dealing with uncertanties/simplifications	 Which uncertanties are the most important to analyse in a sensitivity analysis? How can simplifications be made without increasing uncertanties in results too much?
A) Methodolgy and geotechnics	 Tailings characteristics, foundation Runout volume Impact from hydrology: reservoir volume, lakes downstream, flow scenarios
B) Dam breach modelling	 Definition of tailings rheological properties Breach parameters and development Model tools, domain etc.

1. Dealing with uncertainties and simplifications

A) Methodolgy and geotechnics

- Tailings characteristics, foundation
- Runout volume
- Impact from hydrology: reservoir volume, lakes downstream, flow scenarios
- How to define geotechnical parameters in case of scarce data?
- What can influence the degree of saturation within the TSF?
- Which kind of assumptions can be overly conservative?
- What are the limitations of the different methods available to estimate runout volume?
- What should be taken into account to define relevant hydrological scenarios?

1. Dealing with uncertainties and simplifications

B) Dam breach modelling

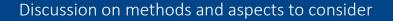
- Definition of tailings rheological properties
- Breach parameters and development
- Model tools, domain etc
- How to define tailings rheological properties to be used in TDBAs?
- What are the advantages and drawbacks of parametric breach parameters estimation methods vs. advanced numerical breach modelling?
- What are the advantages and drawbacks of 2D vs. 3D/CFD models for runout analysis?
- How to handle complex hydrological conditions downstream of the TSF (mixing, temporary blockages...)?
- What are the main uncertainties involved for geo-mechanical modelling of slump failures for identifying runout extents?

2. Credible failure mode

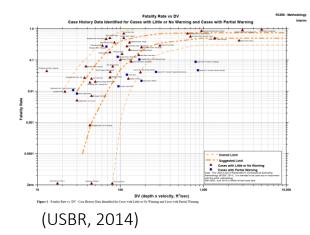
Credible failure mode

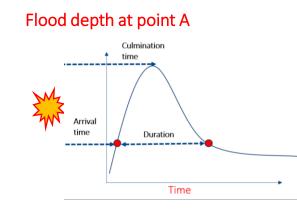
- Define credible failure mode (CFM)?
- Is CFM is the correct term? Do you use any other terms for same purposes
- Does all technically feasible PFM be consider as CFM?
- Does consequence and likelihood in addition to technical feasibility be considered to define CFM?
- Does a PFM with very low likelihood be considered CFM?
- If there is no CFM in a TSF do you need a TDBA
- Do you need an emergency plan for a TSF with no CFM?
- How does CFM used for the risk assessment purposes?
- Does your local regulator defines CFM or set expectation for CFM?

Part 2 Group discussions 3. Consequences - Estimating potential loss of life



- What are the advantages/drawbacks of available methods to estimate potential loss of life?
- What are the most important aspects to consider when estimating potential loss of life due to failure in a tailings facility?
- When is it reasonable to take credit of available warning systems when estimating potential loss of life?
- How do you assess the risk for temporary or permanent work force on the tailings dam or in the facility?





DV (m ² s ⁻¹)	Infants, small children (H.M ≤ 25) and frail/older persons	Children (H.M = 25 to 50)	Adults (H.M > 50)
0	Safe	Safe	Safe
0 - 0.4		Low Hazard ¹	
0.4 - 0.6		Significant Hazard; Dangerous to most	Low Hazard ¹
0.6 - 0.8	Extreme Hazard; Dangerous to all		Moderate Hazard; Dangerous to some ²
0.8 - 1.2		Extreme Hazard; Dangerous to all	Significant Hazard; Dangerous to most ³
> 1.2			Extreme Hazard; Dangerous to all

(ARR, 2010)

Part 2: Group discussions4. Open questions / discussions



Round-up and conclusion



• Summary of group discussions

• Conclusion

